

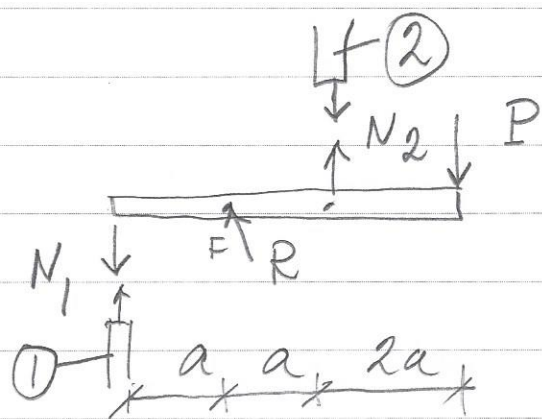
1(9)

Hållfasthetslära 72

2014-05-26

1) Se LB p 25-27

Fritägg den stela balken, inför stängkrafter

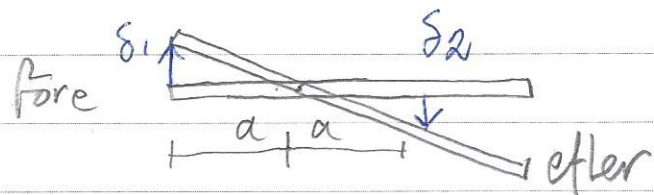


Jämvikt

$$\sum \mathcal{M} N_1 \cdot a + N_2 \cdot a - P \cdot 3a = 0 \quad 1)$$

Deformationer

likformiga trianglar \Rightarrow



$$\frac{\delta_1}{a} = \frac{\delta_2}{a} \quad 2)$$

Materialsamband (LB p27), samma längd L

$$\delta_1 = \frac{N_1 L}{EA} \quad 3)$$

$$\delta_2 = \frac{N_2 L}{EA} \quad 4)$$

2(9)

Hållfasthetslära 72 2014-05-26

1) Ports 3) & 4) i 2) $\Rightarrow N_1 = N_2$; 1) \Rightarrow

$$2 N_1 a = 3 P a \rightarrow N_1 = \frac{3}{2} P$$

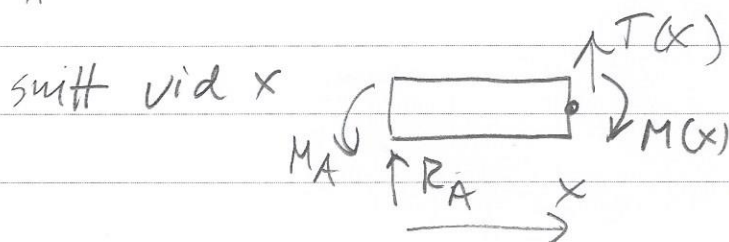
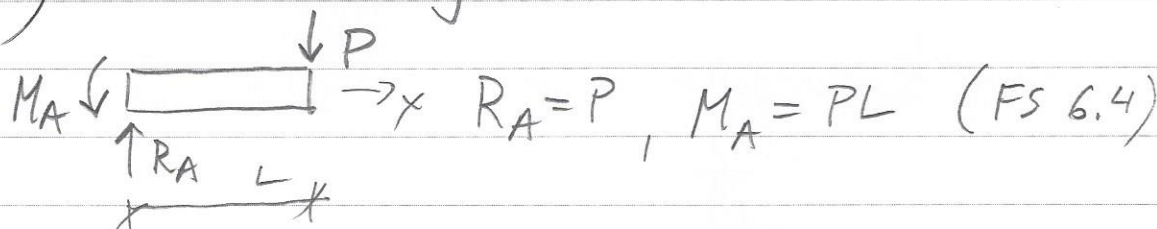
varmed $\delta_1 = \frac{3 PL}{2 EA}$

Vinkeländring $\theta \approx \frac{\delta_1}{a} = \frac{3}{2} \frac{PL}{EAa}$

3(9)

Hållfasthetslära Z2 2014-05-26

2) För balken gäller

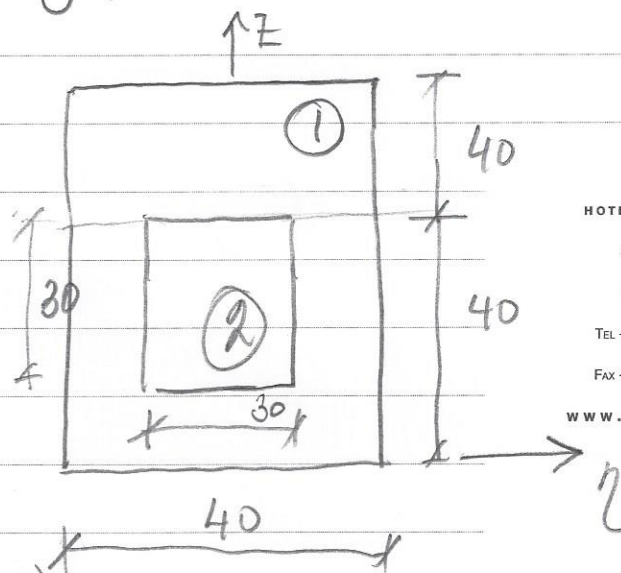


$$\curvearrowleft M_A - R_A x - M(x) = 0$$

$$M(x) = M_A - R_A x = PL - Px$$

Tvärsnittets TP och I_y , se LB p 80-82

$$z_{tp} = \frac{z_1 A_1 - z_2 A_2}{A_1 - A_2} = \frac{40 \cdot 40 \cdot 80 - 25 \cdot 30 \cdot 30}{4080 - 30 \cdot 30}$$



$$z_{tp} = 45,9 \text{ mm}$$

(räknat från underkant)

4(9)

Hållfasthetslära z2 2014-05-26

2.) forts

$$I_y = I_{y1} - I_{y2} = \left(\frac{40 \cdot 80^3}{12} + 40 \cdot 80 \cdot 59^2 \right) - \left(\frac{30 \cdot 30^3}{12} + 30 \cdot 30 \cdot 20,9^2 \right) = 1,36 \cdot 10^6 \text{ mm}^4$$

Böjspänningen $\sigma(x) = \frac{M(x) \cdot z}{I_y}$, $M(x)_{\text{max}} = PL (x=0)$

se LB p 79-80

Drag, ovansida $z_{\text{max}} = 80 - 45,9 = 34,1 \text{ mm}$

Tryck, undersida $z_{\text{min}} = -45,9 = -45,9 \text{ mm}$

$$\Rightarrow \sigma_{\text{tilld}} = 120 = \frac{P \cdot 10^3 \cdot 34,1}{1,36 \cdot 10^6} \rightarrow P = 4,78 \cdot 10^3 \text{ N}$$

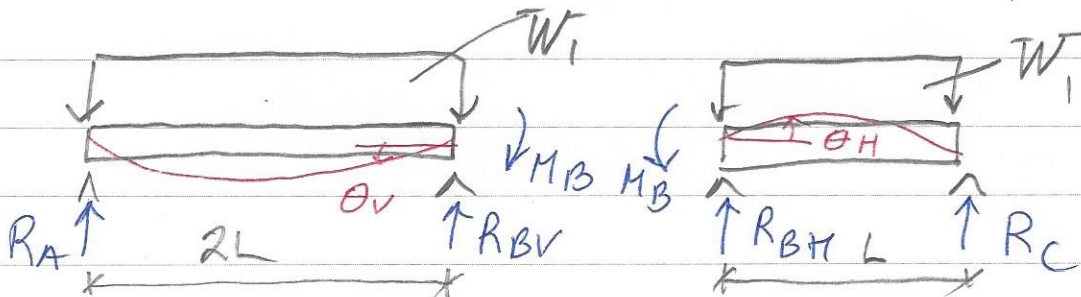
$$\sigma_{\text{tillt}} = -150 = \frac{-P \cdot 10^3 \cdot 45,9}{1,36 \cdot 10^6} \rightarrow P = 4,44 \cdot 10^3 \text{ N}$$

→ Tryck dimensionerande

$$P_{\text{till}} = 4,44 \cdot 10^3 \text{ N}$$

Hållfasthetslära Z2 2014-05-26

3) Dela balken vid mittstödet (LB p 98)



Inför M_B , Använd elementarfäll FS 6.3, $W_1 = \frac{Q}{3L}$

$$\theta_V = \frac{Q(2L)^3}{3L \cdot 24EI} - \frac{M_B 2L}{3EI}$$

$$\theta_H = -\frac{Q}{3L} \frac{L^3}{24EI} + \frac{M_B L}{3EI}$$

$$\theta_V = \theta_H \Rightarrow \frac{Q}{3L} \cdot \frac{L^3}{24EI} (8+1) = \frac{M_B L}{3EI} (1+2)$$

$$\rightarrow M_B = \frac{QL}{8}$$

Stödreaktioner

$$R_A = \frac{Q}{3L} \cdot \frac{2L}{2} - \frac{QL}{8 \cdot 2L} = \frac{13}{48} Q$$

$$R_{BV} = \frac{Q}{3L} \cdot \frac{2L}{2} + \frac{QL}{8 \cdot 2L} = \frac{19}{48} Q$$

6(9)

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3) forts $R_{BH} = \frac{Q}{3L} \cdot \frac{L}{2} + \frac{QL}{8L} = \frac{14}{48} Q$

$R_C = \frac{Q}{3L} \cdot \frac{L}{2} - \frac{QL}{8L} = \frac{2}{48} Q$

$\rightarrow R_B = R_{BV} + R_{BH} = \frac{33}{48} Q$

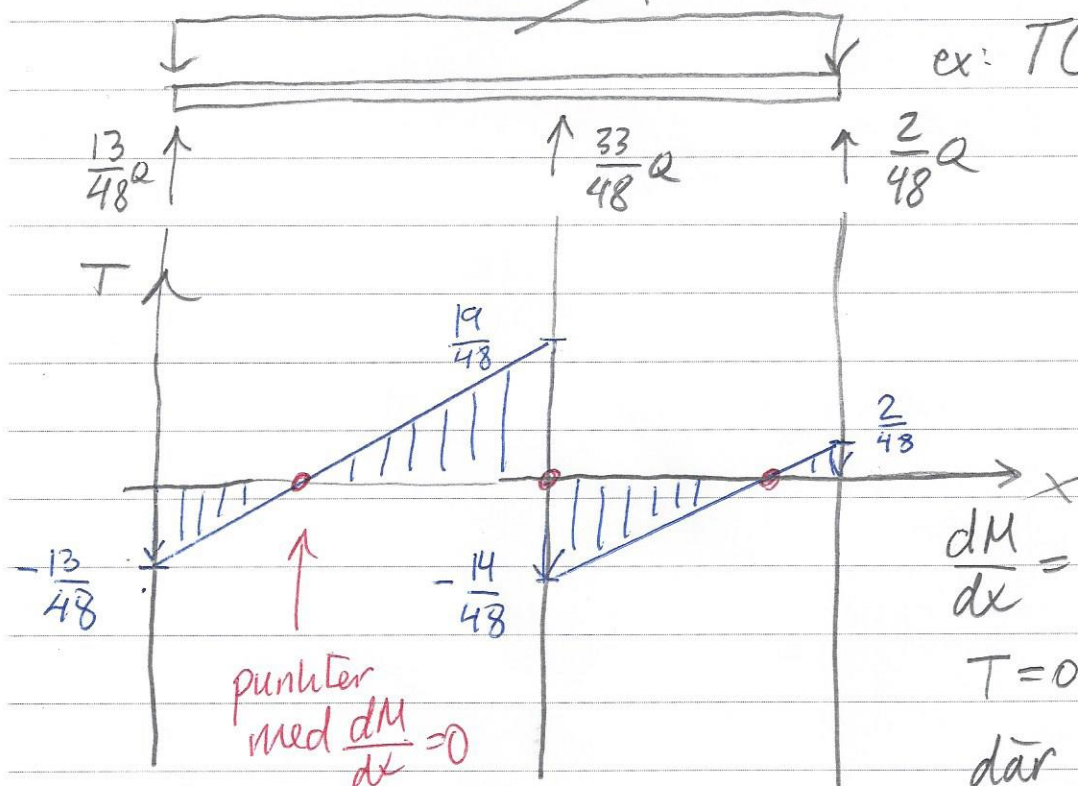
Rita T-diagram. LB p 70-72 \Rightarrow

$\frac{dT}{dx} = -q(x) \Rightarrow T(x_2) - T(x_1) = - \int_{x_1}^{x_2} q(x) dx$

$q(x) = W_1 = \frac{Q}{3L}$

ex: $T(2L) - T(0^+) =$

$= - \left(- \frac{Q}{3L} \cdot 2L \right)$
 $= \frac{32}{48} Q$



punkter med $\frac{dM}{dx} = 0$

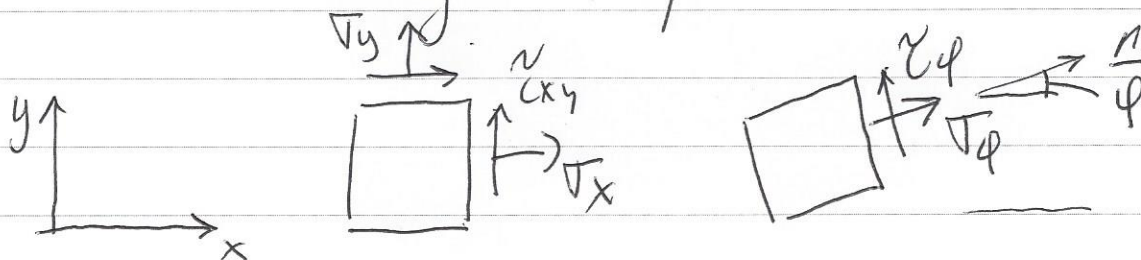
$\frac{dM}{dx} = T \Rightarrow$

$T=0$ är lägen där $M(x)$ kan vara

7(9)

Hållfasthetslära Z1 2014-05-26

4 Vi har enligt LB p 170-171



$$\sigma_\phi = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\phi + \tau_{xy} \sin 2\phi$$

$$\rightarrow \sigma_\phi = \frac{5-8}{2} + \frac{5-(-5)}{2} \cos 2\phi + 7 \sin 2\phi$$

$$\sigma_\phi = 0 \Rightarrow \tan 2\phi = \frac{-5}{7} \rightarrow \phi = -18^\circ$$

$$\text{då läs } \tau_\phi = -\frac{\sigma_x - \sigma_y}{2} \sin 2\phi + \tau_{xy} \cos 2\phi =$$

$$= -\frac{5-(-5)}{2} \sin(-35.5^\circ) + 7 \cos(-35.5^\circ) = 8.6 \text{ MPa}$$

τ_ϕ då $\sigma_\phi = 0$ är också radien i Mohrs cirkel
LB p 175

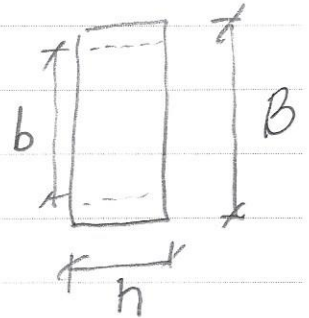
$$\tau_\phi = \tau_{\max} = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2} = 8.6 \text{ MPa}$$

Hållfasthetslära 72 2014-05-26

Stångens tvärsnittsarea $A = h \cdot b$

$$\rightarrow \sigma = \frac{F_0}{A} + \frac{F_0}{2A} \sin \omega t \Rightarrow$$

$$\sigma_m = \frac{250 \cdot 10^3}{40 \cdot 80} = 78 \text{ MPa}, \quad \sigma_a = 39 \text{ MPa}$$



Använd reducerat Haigh diagram LB p 250, 255

λ : ej gjutet $\rightarrow \lambda = 1$

K_d : vi har anvisning $\rightarrow K_d = 1$

$$K_r: \text{LB p 254} \quad \left. \begin{array}{l} R_a = 10 \text{ mm} \\ R_m = \sqrt{B} = 540 \text{ MPa} \end{array} \right\} \Rightarrow \frac{1}{K_r} \approx 0.87$$

$$K_t: \text{LB Fig 159a} \quad \left. \begin{array}{l} \frac{B}{b} = \frac{100}{80} \\ \frac{s}{b} = \frac{10}{80} \end{array} \right\} \rightarrow k_t \approx 2.2$$

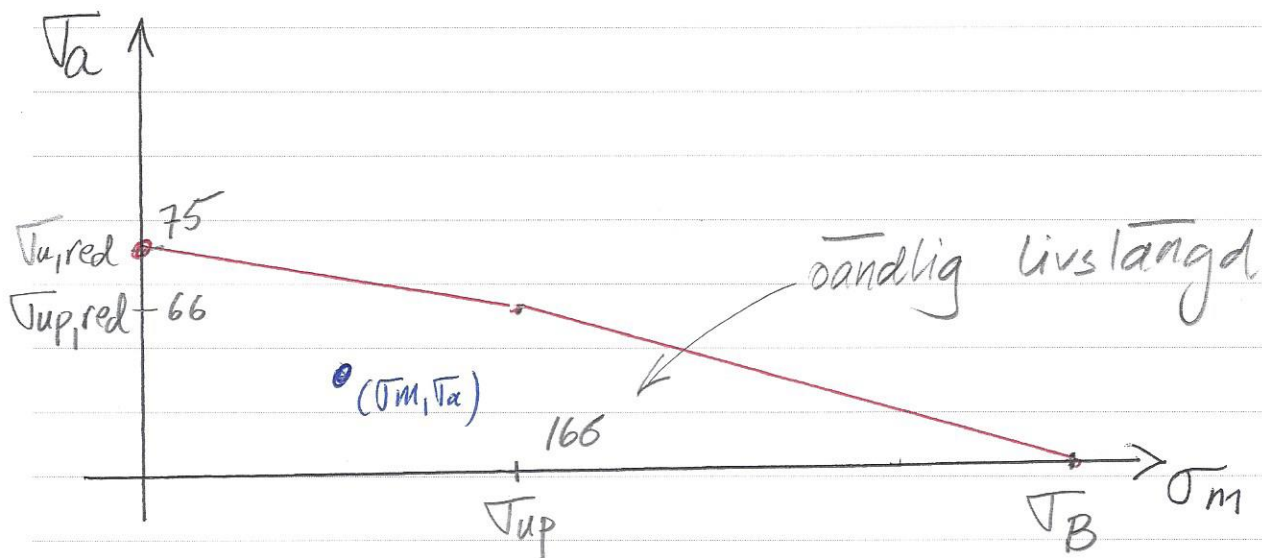
$$\text{Fig 160} \Rightarrow \left. \begin{array}{l} s = 10 \text{ mm} \\ R_m = \sqrt{B} = 540 \end{array} \right\} \Rightarrow q \approx 0.9$$

$$\rightarrow K_f = 1 + q(k_t - 1) = 1 + 0.9 \cdot 1.2 = 2.1$$

$$\sigma_{u,red} = \pm \sigma_u \frac{\lambda}{K_r K_f K_d} = \pm 180 \frac{1 \cdot 0.87}{1 \cdot 2.1} = 75 \text{ MPa}$$

Hållfasthetslära 72 2014-05-26

$$\begin{aligned}\sigma_{up, red} &= 160 \pm 160 \cdot \frac{1 \cdot 0,87}{1 \cdot 2 \cdot 1} = \\ &= 160 \pm 66 \text{ MPa}\end{aligned}$$



\Rightarrow ingen risk för utmattning